

渠務署及城市規劃委員會：

A/YL-KTN/1018 的渠務報告詳細

申請地點範圍有約 1,559.1 平方米，位於錦田北的鄉郊範圍。目前為露天空間及建有臨時建築物。

申請地點附近有大量的臨時建築物及草地。現有水平為約+14.4 mPD (此水平已完成填土及平整)。

有一條自然溪流位於申請地點的東面，並計劃將場內水流引導到該溪流。

申請範圍的東北面水平比申請地點高，有機會會有水流從這面流入。申請範圍北外改有約 450mm 的渠道，因此沒有流水從其他方向流入申請地點。

申請地點的擬議佈局平面圖請參考 Appendix 2。

申請地點範圍有約 1,559.1 平方米，全部將以混凝土作表面，在申請地點外有約 4,115.9 平方米，大多為草地、道路及建築物。

擬議發展	
申請地點範圍 (約 m ²)，全部已以混凝土平整	1,559.1
申請地點外集水區	
申請地點外北面集水區 (約 m ²)，大多為草地、道路及建築物，本報告將以約 93% 混凝土作評估	4,115.9

根據 STORMWATER DRAINAGE MANUAL (SDM) - Table 10 – Recommended Design Return

Periods based on Flood Levels

Intensively Used Agricultural Land	2-5 years
Village Drainage including Internal Drainage System under a Polder Scheme	10 years
Main Rural Catchment Drainage Channels	50 years
Urban Drainage Trunk Systems	200 years
Urban Drainage Branch Systems	50 years

本報告將使用 Main Rural Catchment Drainage Channels, 1 in 50 years return period 作評估。

本渠道設計將參考由 貴署所編寫的 SDM 作基礎，以下為所採用的數據及計算方法。

1. Intensity-Duration-Frequency Relationship - The Recommended Intensity-Duration-Frequency relationship is used to estimate the intensity of rainfall. It can be expressed by the following algebraic equation.

$$i = \frac{a}{(t_d + b)^c}$$

The site is located within the HKO Headquarters Rainfall Zone. Therefore, for 50 years return period, the following values are adopted.

a	=	451.3
b	=	2.46
c	=	0.337

2. The peak runoff is calculated by the Rational Method.

$$Q_p = 0.278 C i A$$

where V	=	peak runoff in m ³ /s
C	=	runoff coefficient (dimensionless)
i	=	rainfall intensity in mm/hr
A	=	catchment area in km ²

3. According to Section 7.5.2(b) of the Stormwater Drainage Manual (SDM), Fifth Edition January 2018

<u>Surface Characteristics</u>	<u>Runoff coefficient, C</u>
Asphalt	0.70-0.95
Concrete	0.80-0.95
Brick	0.70-0.85
Grassland (heavy soil)	
Flat	0.13-0.25
Steep	0.25-0.35
Grassland (sandy soil)	
Flat	0.05-0.15
Steep	0.15-0.20

The run-off coefficient (C) of surface runoff area taken as follows:

- Concrete Area C = 0.95
- Grassland (Heavy soil) with flat surface C = 0.25

4. Manning's Equation is used for calculation of velocity of flow inside the channels. It can be expressed by the following algebraic equation.

$$V = \frac{R^{1/6}}{n} \sqrt{RS_f}$$

- where V = Velocity of the pipe flow (m/s)
 S_f = Hydraulic gradient
 n = manning's coefficient
 R = Hydraulic radius (m)

5. Colebrook-White Equation is used for calculation of velocity of flow inside the pipes. It can be expressed by the following algebraic equation.

$$\bar{V} = -\sqrt{32gRS} \log \log \left(\frac{k_s}{14.8R} + \frac{1.255v}{R\sqrt{32gRS_f}} \right)$$

- where V = Velocity of the pipe flow (m/s)
 S_f = Hydraulic gradient
 k_f = roughness value (m)
 v = kinematics viscosity of fluid
 D = pipe diameter (m)
 R = Hydraulic radius (m)

申請範圍主要平坦，並緩緩斜向東面，渠道設計請參考 Appendix 5。

渠道容量計算請參考 Appendix – Calculation。

根據本報告，本臨時發展不會對附近的渠道有重大影響。

二零二四年七月二十日

Check The Capacity of Existing Natural Stream

Manning Equation is used in hydraulic design and analysis. The cross-sectional mean velocity is given in the following expression:

$$V = \frac{R^{1/6}}{n} \sqrt{RS_f}$$

Where R = hydraulic (m)
N = Manning coefficient (s/m^{1/3}), refer Table 13 of SDM
Sf = friction gradient (dimensionless)

Using Manning's Equation

$$V = R^{2/3} * S_f^{0.5} / n$$

Where R = A/P = 0.667 m A = 4 m²
P = 6 m
n = 0.05 s/m^{1/3} (Table 13 of Stormwater Drainage Manual)
Sf = 0.245

Therefor V = 0.667^{2/3}*0.245^{0.5}/0.05
= 7.55 m/sec

Maximum Capacity (Qmax)
= V*A
= 30.2 m³/sec
> Q_{total} *Allowed 10% for situation.

The Existing Natural Stream has enough capacity.

Appendix – Calculation

Capacity Flows Estimation for Propose Catchments and Drainage System with 50 Year Return Period

A1. Calculation of On-Site Runoff (After Development)

Surface Type	Catchment Area (A), m ²	Catchment Area (A), km ²	Average slope (H), m/100m	Flow path length (L), m	Inlet time (t ₀), min	Time of Concentration (t _c), min	Duration (t _d), min	a (50 year return period)	b (50 year return period)	c (50 year return period)	Runoff intensity (i) mm/hr	Runoff coefficient (C)	C x A	Peak runoff (Q _p), m ³ /s
7% Grassland (Heavy soil) with flat surface + 93% Concrete	5,675	0.005675	3.52	96.7	4.58	4.58	4.58	451.3	2.46	0.337	234	0.9	0.0051075	0.332
Total													0.332	

A2. Calculation of the Capacity of Proposed Drainage (After Development)

Channel Type	Width, m	Depth, m	Slope	Length, m	Manning's Roughness Coefficient	Cross Section Area, m ²	Wetted Perimeter, m	Hydraulic radius, m	Mean Velocity, m/s	Capacity flow, m ³ /s	Catchment Served, km ²	Runoff, m ³ /s	% of capacity flow	Sufficient Capacity (Y/N)
Concrete Channel	0.45	0.45	0.005	203	0.015	0.26	1.157	0.225	1.74	0.453	0.005675	0.332	73%	Y

*Allowed 10% for siltation

Note:

Runoff is calculated in accordance with DSD's "Stormwater Drainage Manual – Planning, Design and Management" (SDM), fifth edition, January 2018.

Equation used: $t_0 = \frac{0.14465L}{H^{0.2}A^{0.1}}$ $t_c = t_0 + t_f$ $i = \frac{a}{(t_d+b)^c}$ $Q_p = 0.278 C i A$ $V = \frac{R^{1/6}}{n} \sqrt{RS_f}$

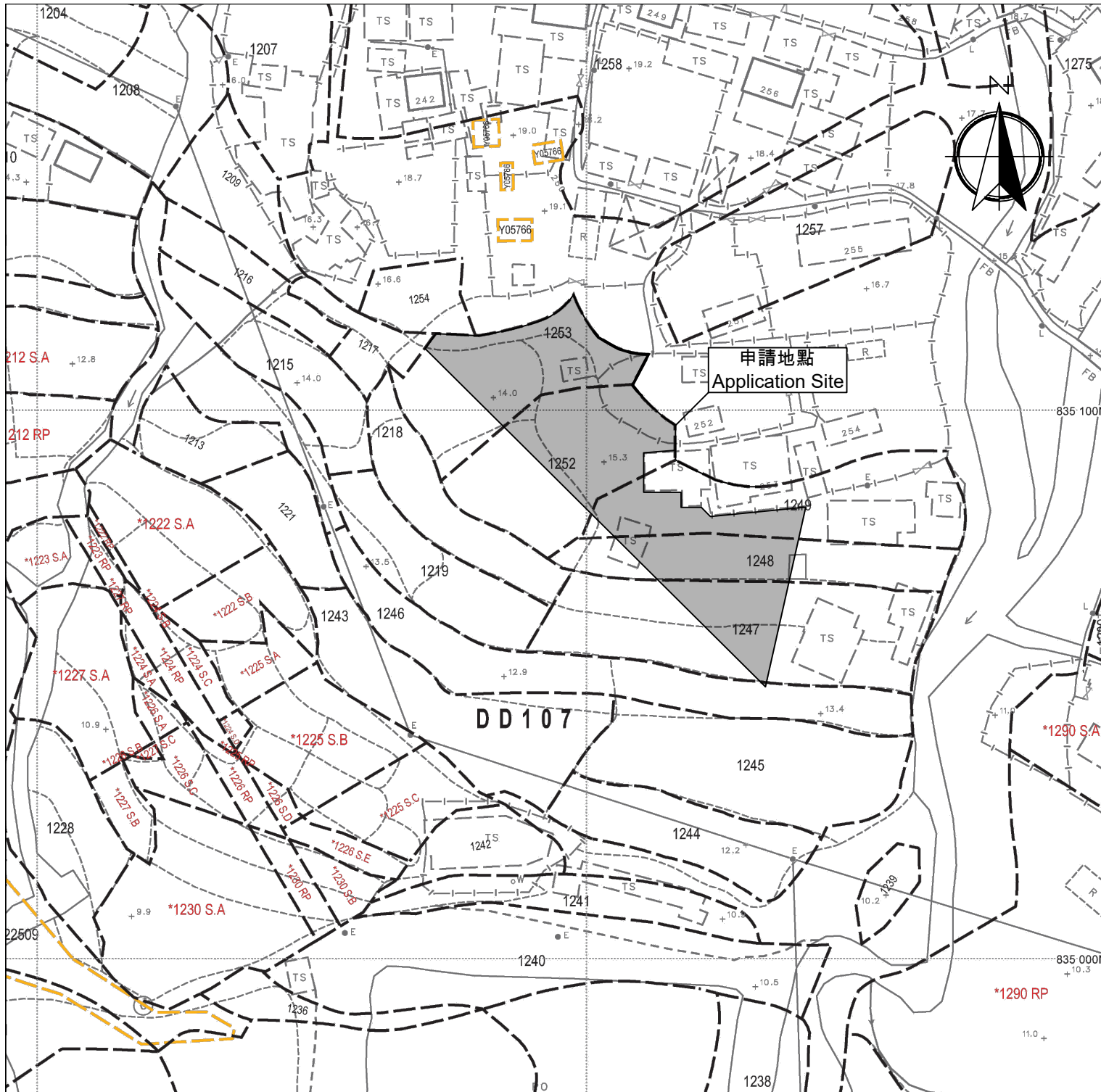
B1. Calculation of the runoff of Existing Drainage System

Surface Type	Catchment Area (A), m ²	Catchment Area (A), km ²	Average slope (H), m/100m	Flow path length (L), m	Inlet time (t ₀), min	Time of Concentration (t _c), min	Duration (t _d), min	a (50 year return period)	b (50 year return period)	c (50 year return period)	Runoff intensity (i) mm/hr	Runoff coefficient (C)	C x A	Peak runoff (Q _p), m ³ /s
15% Concrete + 85% Grassland (Heavy soil) with steep surface	545,522	0.545522	24.5	1,470	29.93	29.93	29.93	451.3	2.46	0.337	140	0.44	0.24003	9.34
Total													9.34	

B2. Adequacy Check for Existing Drainage System

Channel Type	Width, m	Depth, m	Slope	Length, m	Manning's Roughness Coefficient	Cross Section Area, m ²	Wetted Perimeter, m	Hydraulic radius, m	Mean Velocity, m/s	Capacity flow, m ³ /s	Catchment Served, km ²	Runoff, m ³ /s	% of capacity flow	Sufficient Capacity (Y/N)
Natural-Stream (7)	2	2	0.245	1,470	0.05	4	6	0.667	7.55	30.2	0.530231	9.34	31%	Y

*Allowed 10% for siltation. For assessment purpose, assume width and depth of the channel is 2m.



Legend:

Application Site 申請範圍

Appendix 1

Location: DD 107 Lot 1247 (Part),
1248 (Part), 1249 (Part), 1252 (Part)
and 1253 (Part)

OZP: S/YL-KTN/11
District: Kam Tin North
Zoning: Agriculture

Date: 23 April 2024

Location

位置圖

擬議臨時露天存放建築機械及物料
(為期3年)及填土

Proposed Temporary Open Storage of
Construction Machinery and Materials
For a Period of 3 Years and Filling of Land

SCALE

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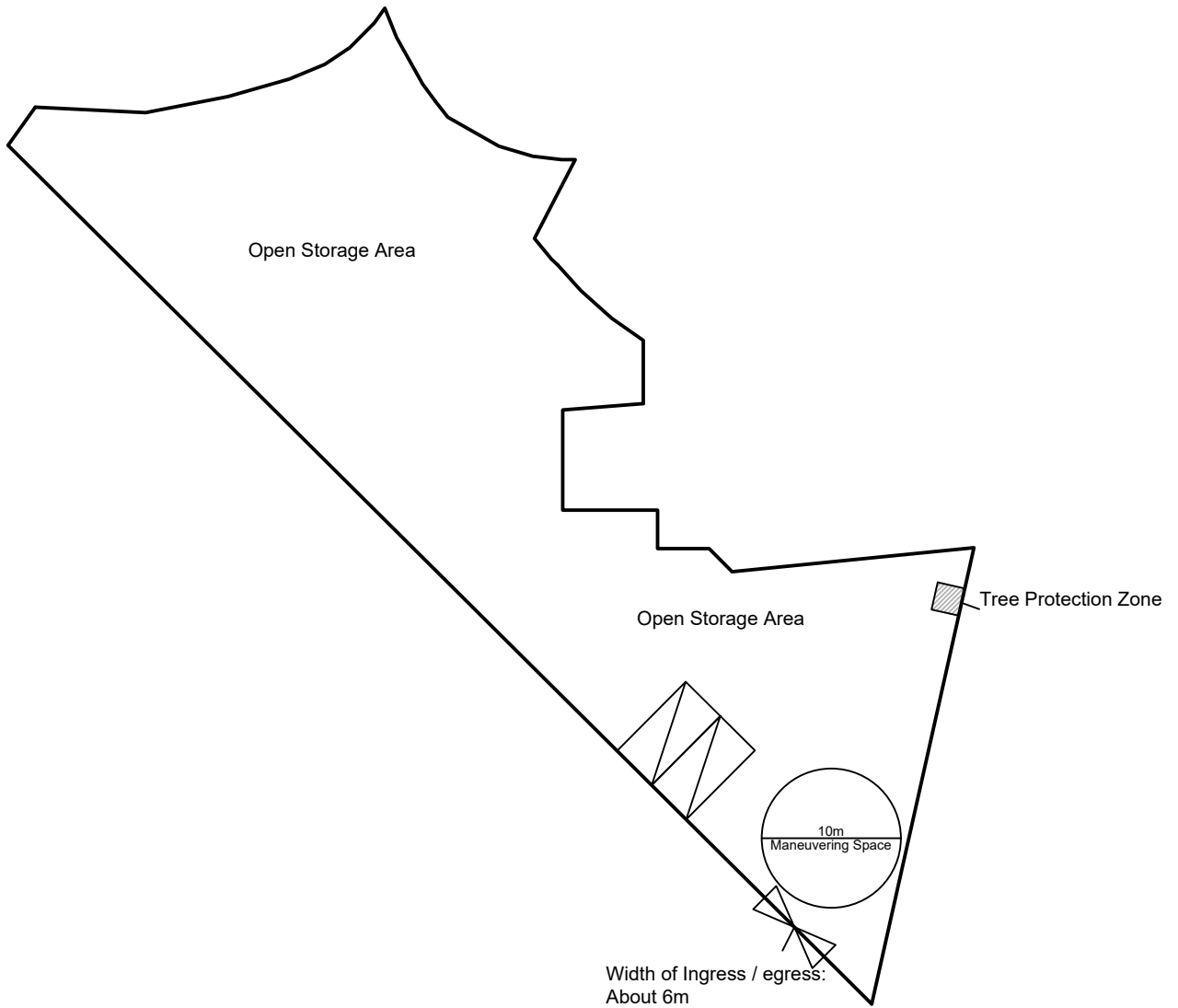
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For Identification Only

Drawing No.:

1-01

LGV L/UL Space
 Dimension: 7m x 3.5m
 Unit(s): 2



Legend:

- Ingress/egress (Width: About 6m)
- LGV L/UL Space (Unit(s): 2)
- Maneuvering Space

Total Area: 1,559.1 m² (About)

Appendix 2

Location: DD 107 Lot 1247 (Part),
 1248 (Part), 1249 (Part), 1252 (Part)
 and 1253 (Part)

OZP: S/YL-KTN/11
 District: Kam Tin North
 Zoning: Agriculture

Proposed Layout Plan

擬議佈局平面圖

擬議臨時露天存放金屬、膠喉、機器、
 汽車零件及建築材料(為期3年)及填土

Proposed Temporary Open Storage of Metal, Plastic
 Pipes, Machinery, Vehicle Parts and Construction
 Materials

For a Period of 3 Years and Filling of Land

SCALE

1:500

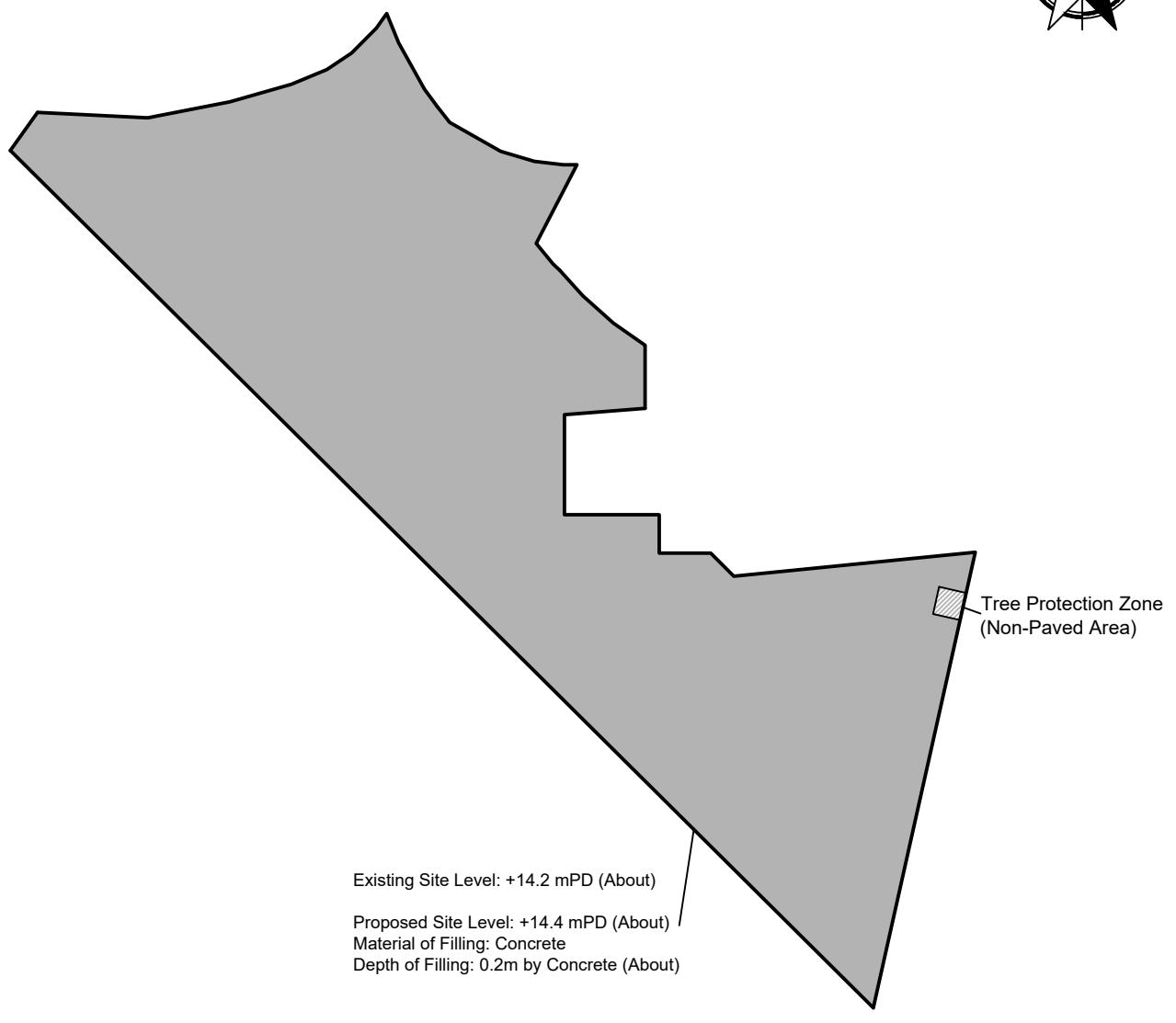
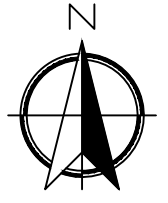
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For Identification Only

Drawing No.:

Date: 16 September 2024

2-01





Total Area: 1,559.1 m² (About)

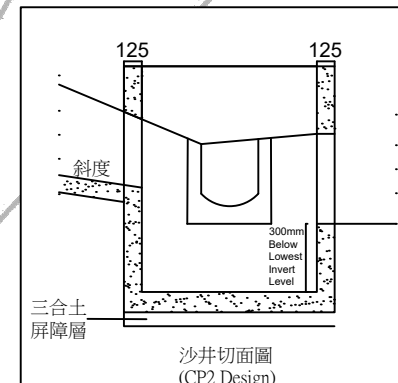
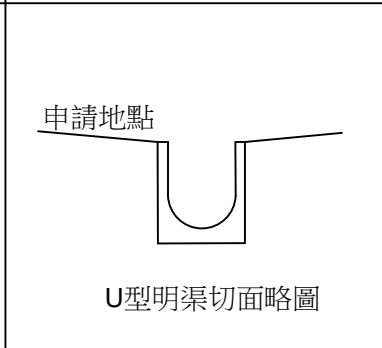
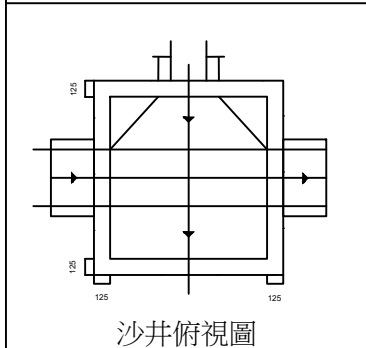
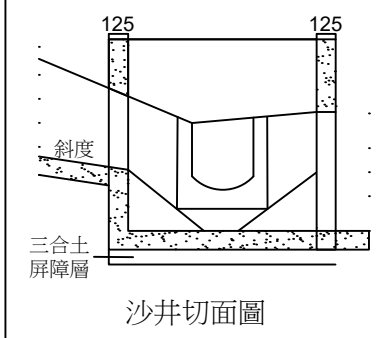
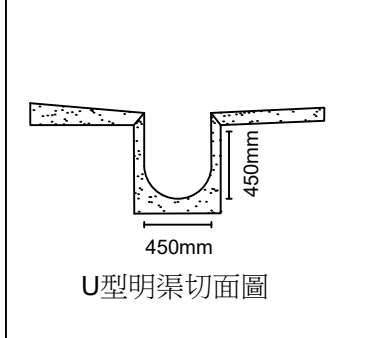
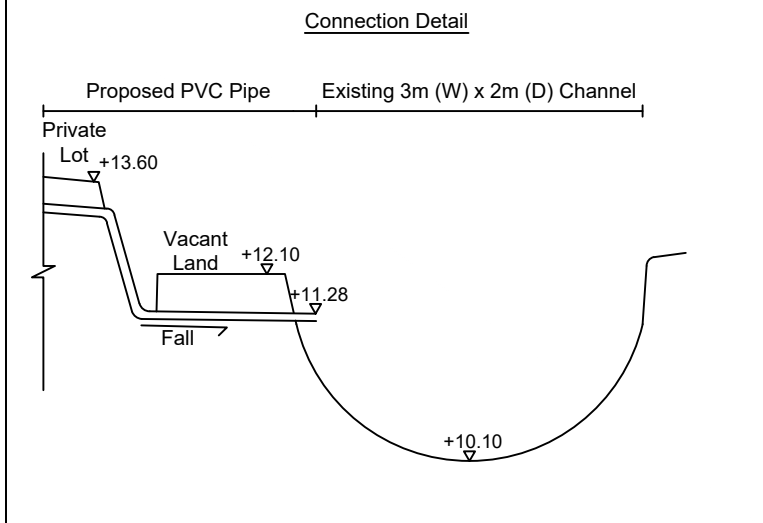
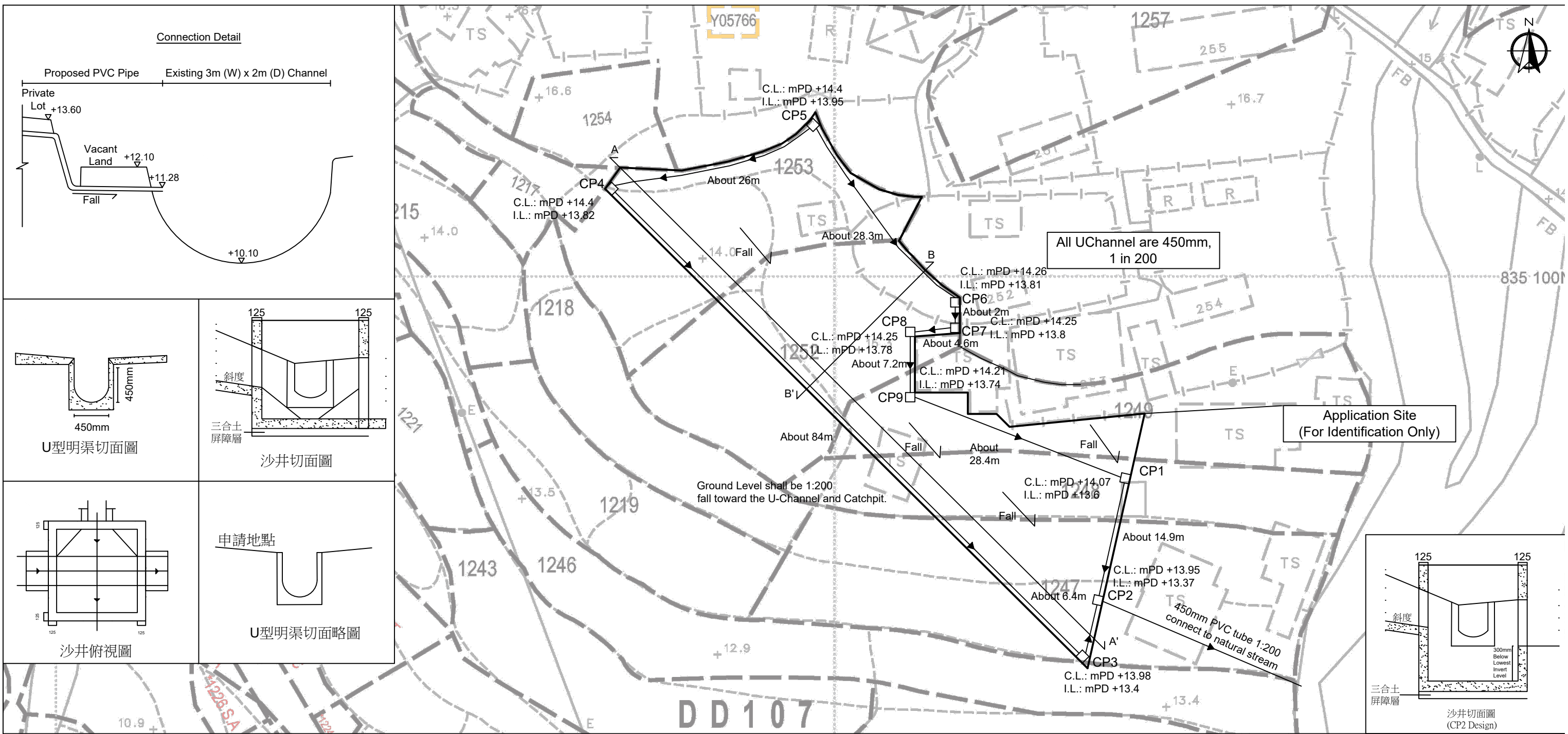
Paved Area: 1,555.1 m² (About)

Non-Paved Area: 4 m² (About)

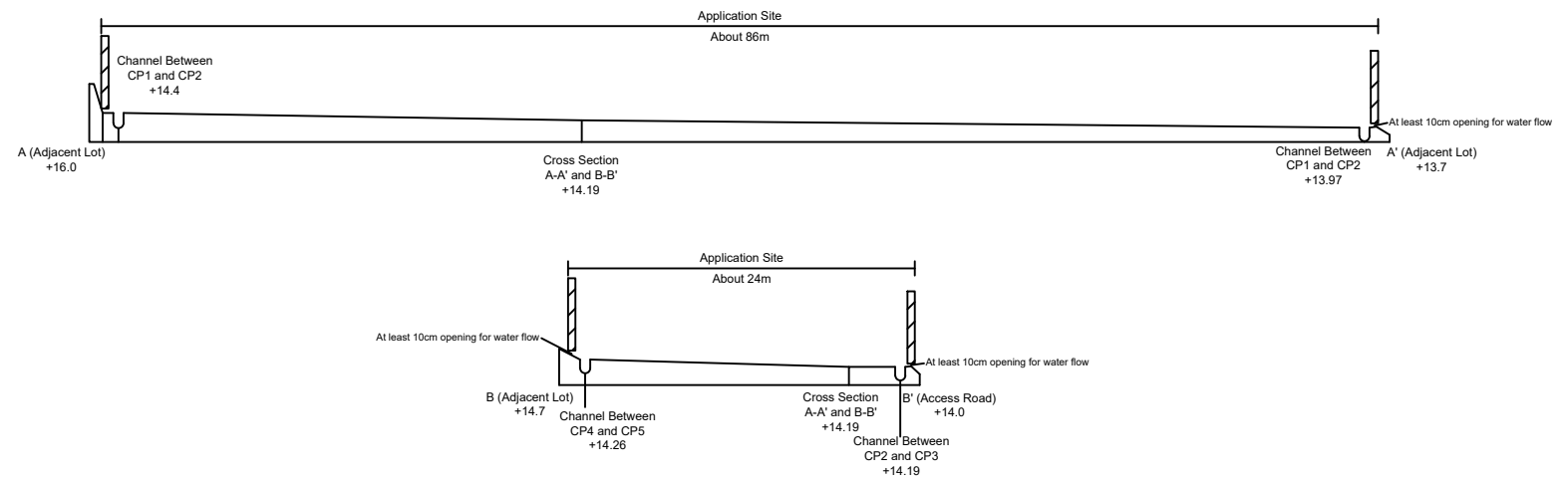
Legend:

-  Paved Area 平整範圍
-  Non-Paved Area 不平整範圍

<p align="center"><u>Appendix 4</u></p> <p>Location: DD 107 Lot 1247 (Part), 1248 (Part), 1249 (Part), 1252 (Part) and 1253 (Part)</p> <p>OZP: S/YL-KTN/11 District: Kam Tin North Zoning: Agriculture</p> <p>Date: 15 September 2024</p>	<p align="center"><u>Paved Area</u> 平整位置圖</p> <p align="center">擬議臨時露天存放建築機械及物料 (為期3年)及填土</p> <p align="center">Proposed Temporary Open Storage of Construction Machinery and Materials For a Period of 3 Years and Filling of Land</p>	<p align="center"><u>SCALE</u></p> <p align="center">1:500</p> <p align="center">@A4</p>	
		<p align="center">For Identification Only</p>	<p align="center">Drawing No.: 4-01</p>



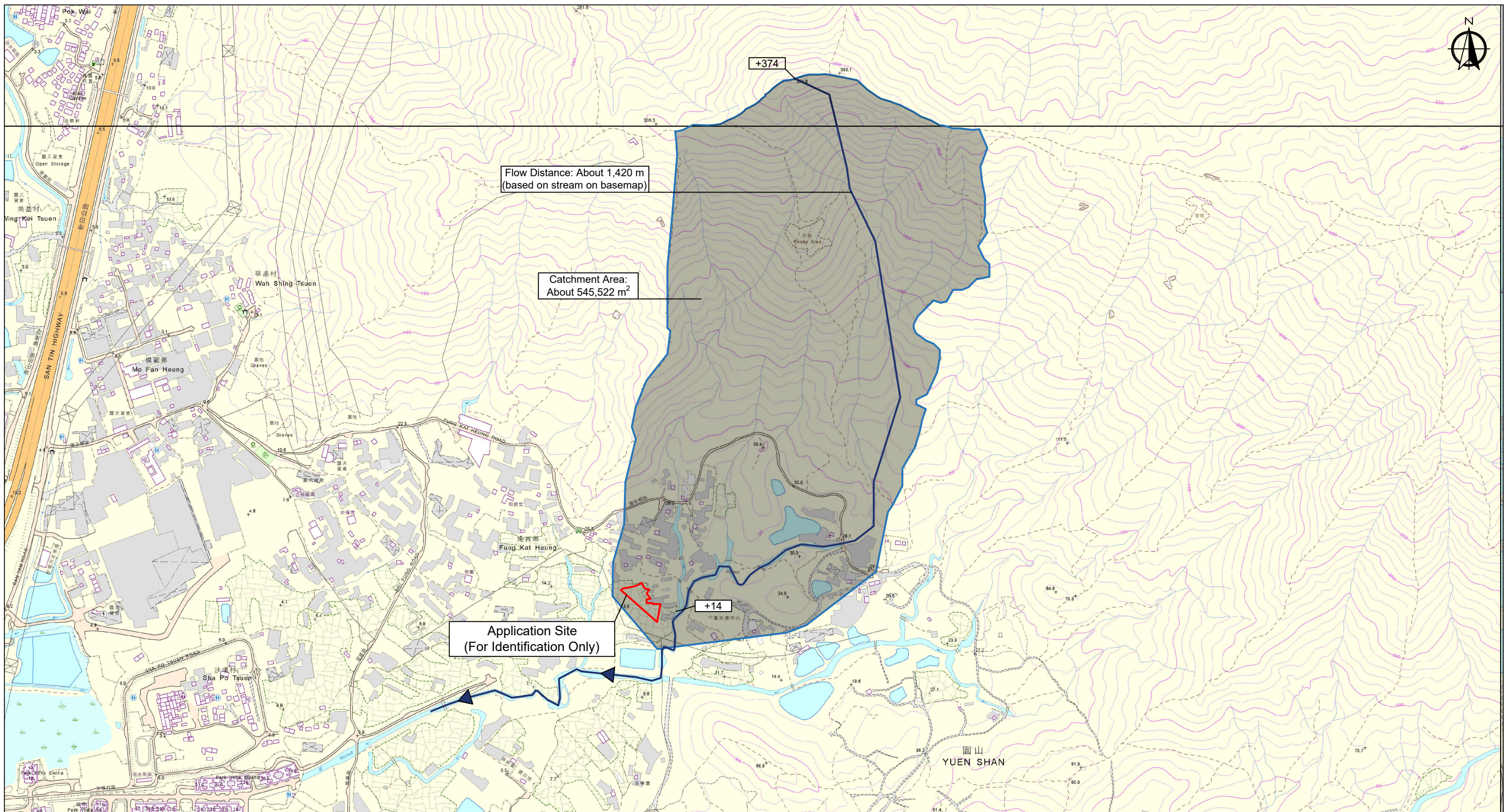
- Note:
1. Adequate opening will be provided around the application site.
 2. Catchpit design shall follow CEDD standard drawing No. C24061.
 3. All proposed U-channel and Catchpit must maintain in good shape (i.e. Inspection and maintenance regularly).
 4. Grating Cover is provided to reduce the irregular road surface from entering the site.



- Legend:
- Proposed Catchpit
 - Proposed U-Channel
 - ▶ Water Flow

*Invert Level of Connection Point Should Be Verified On Site Before Construction.
 *Cover Level Are Indicative Only Which Should Be Verified On Site.

Appendix 5	Location: DD 107 Lot 1247 (Part), 1248 (Part), 1249 (Part), 1252 (Part) and 1253 (Part) OZP: S/YL-KTN/11 District: Kam Tin North Zoning: Agriculture	Project: Proposed Temporary Warehouse (excluding Dangerous Goods Godown) with Ancillary Facilities for a Period of 3 Years and Filling of Land	Proposed Drainage Plan and Cross Section	Scale: 1:500 @A4	Drawing No.
					5-01
					For Identification Only
					Date: 19 July 2024



Flow Distance: About 1,420 m
(based on stream on basemap)

Catchment Area:
About 545,522 m²

Application Site
(For Identification Only)

Captured from hkmapservices iB5000 2-SE-C and iB5000 6-NE-A on 6th July 2024
Calculation Please refer to Appendix - Capacity

Legend:

- Catchment Area
- Existing Channel
- Application Site

Appendix 5.1

Location: DD 107 Lot 1247 (Part),
1248 (Part), 1249 (Part), 1252 (Part)
and 1253 (Part)
OZP: S/YL-KTN/11
District: Kam Tin North
Zoning: Agriculture

Project:
Proposed Temporary Warehouse (excluding Dangerous Goods
Godown) with Ancillary Facilities for a Period of 3 Years and Filling
of Land

Catchment Area of Existing Channel

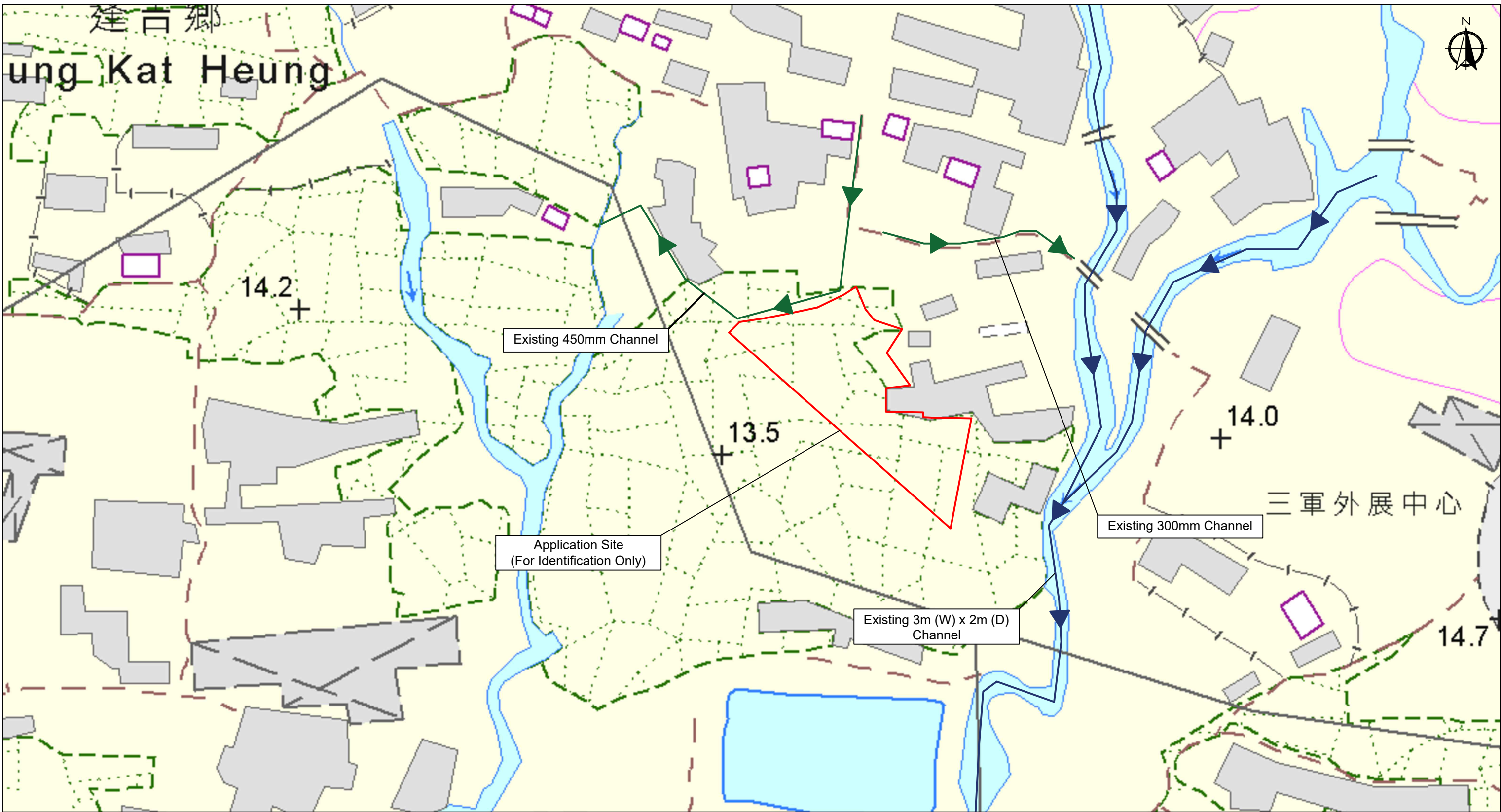
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Drawing No.

5.1-1

For Identification Only

Date: 10 July 2024



- Legend:
- Existing Channel
 - Existing Channel
 - Application Site

Captured from hkmappointments ib5000 2-SE-C and ib5000 6-NE-A on 6th July 2024

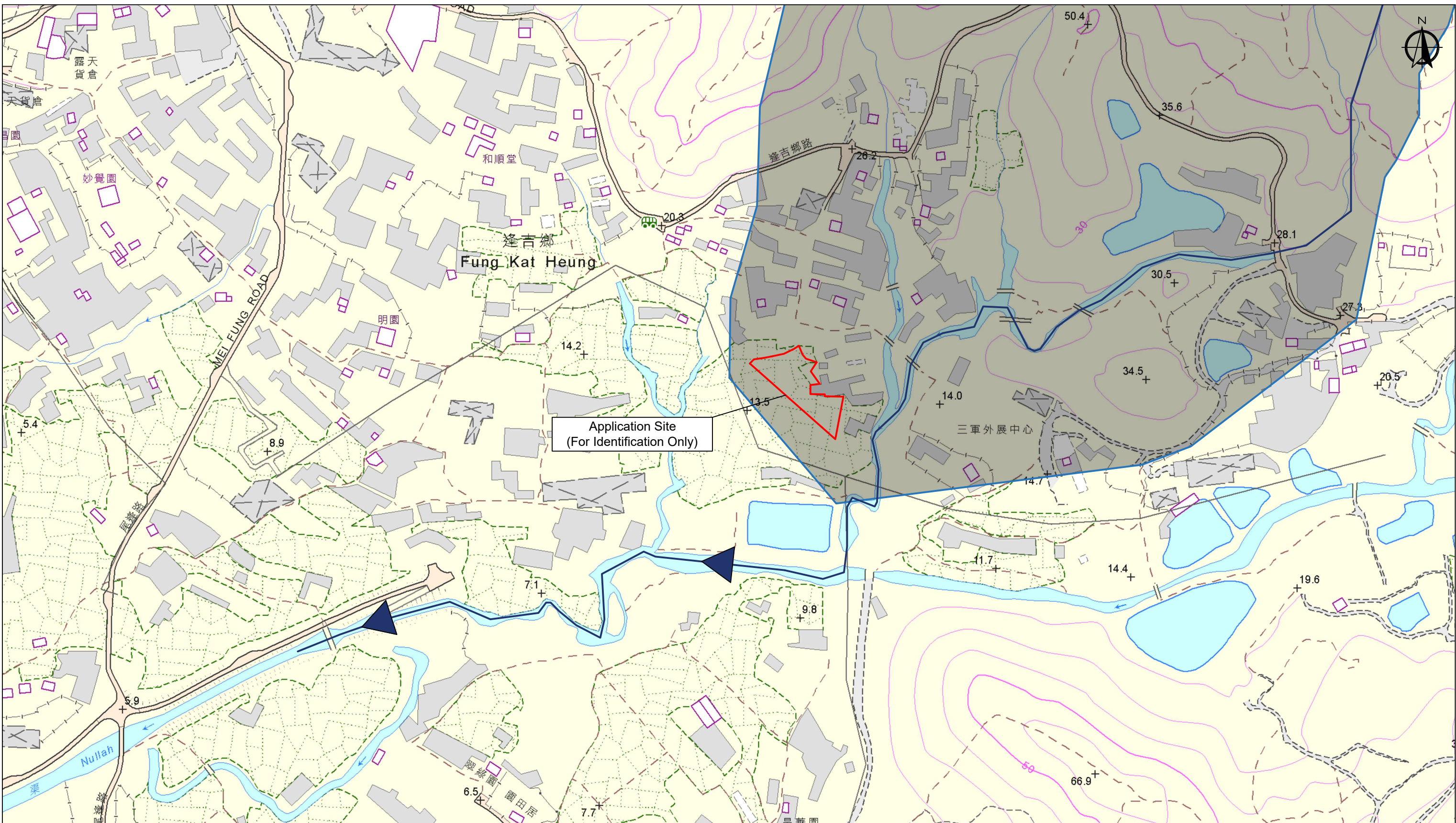
Appendix 5.3	Location: DD 107 Lot 1247 (Part), 1248 (Part), 1249 (Part), 1252 (Part) and 1253 (Part) OZP: S/YL-KTN/11 District: Kam Tin North Zoning: Agriculture	Project: Proposed Temporary Warehouse (excluding Dangerous Goods Godown) with Ancillary Facilities for a Period of 3 Years and Filling of Land	Existing Drainage Channel Plan	Scale: Undefined @A4	Drawing No. 5.3-1 For Identification Only Date: 19 July 2024



- Legend:
- Existing Channel
 - Existing Channel
 - Application Site

Captured from hkmappointments ib5000 2-SE-C and ib5000 6-NE-A on 6th July 2024

Appendix 5.4	Location: DD 107 Lot 1247 (Part), 1248 (Part), 1249 (Part), 1252 (Part) and 1253 (Part) OZP: S/YL-KTN/11 District: Kam Tin North Zoning: Agriculture	Project: Proposed Temporary Warehouse (excluding Dangerous Goods Godown) with Ancillary Facilities for a Period of 3 Years and Filling of Land	Photos Record of Surroundings and Viewing Point	Scale: Undefined @A4	Drawing No.	
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						For Identification Only
						Date: 19 July 2024



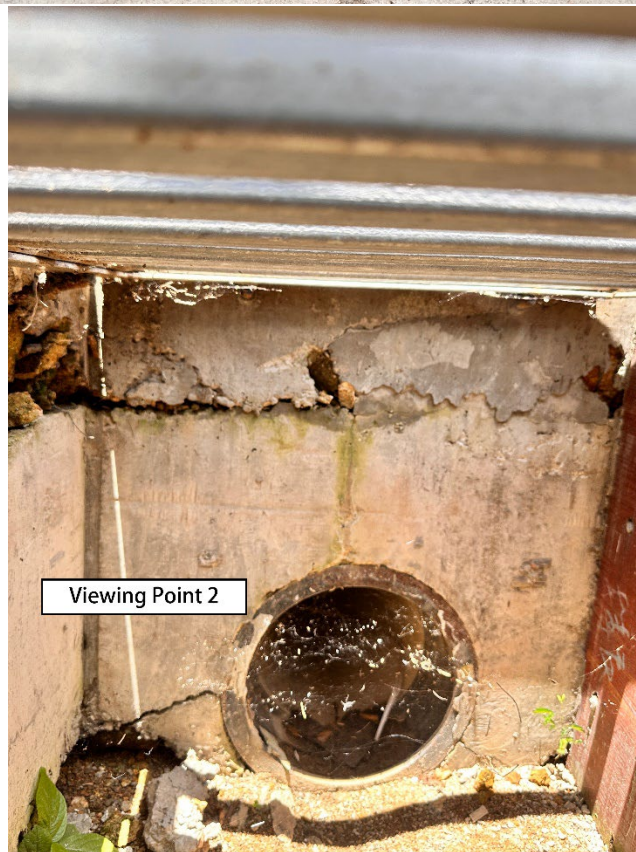
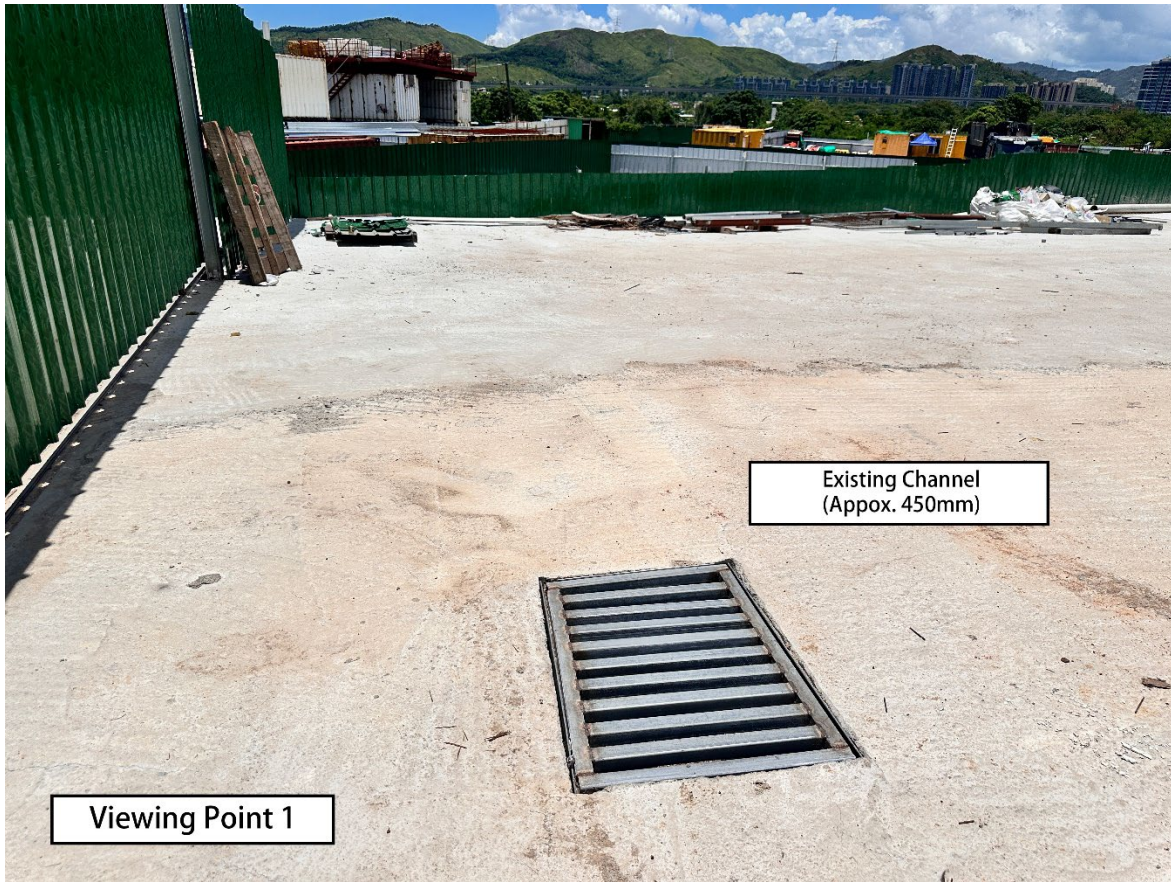
Legend:

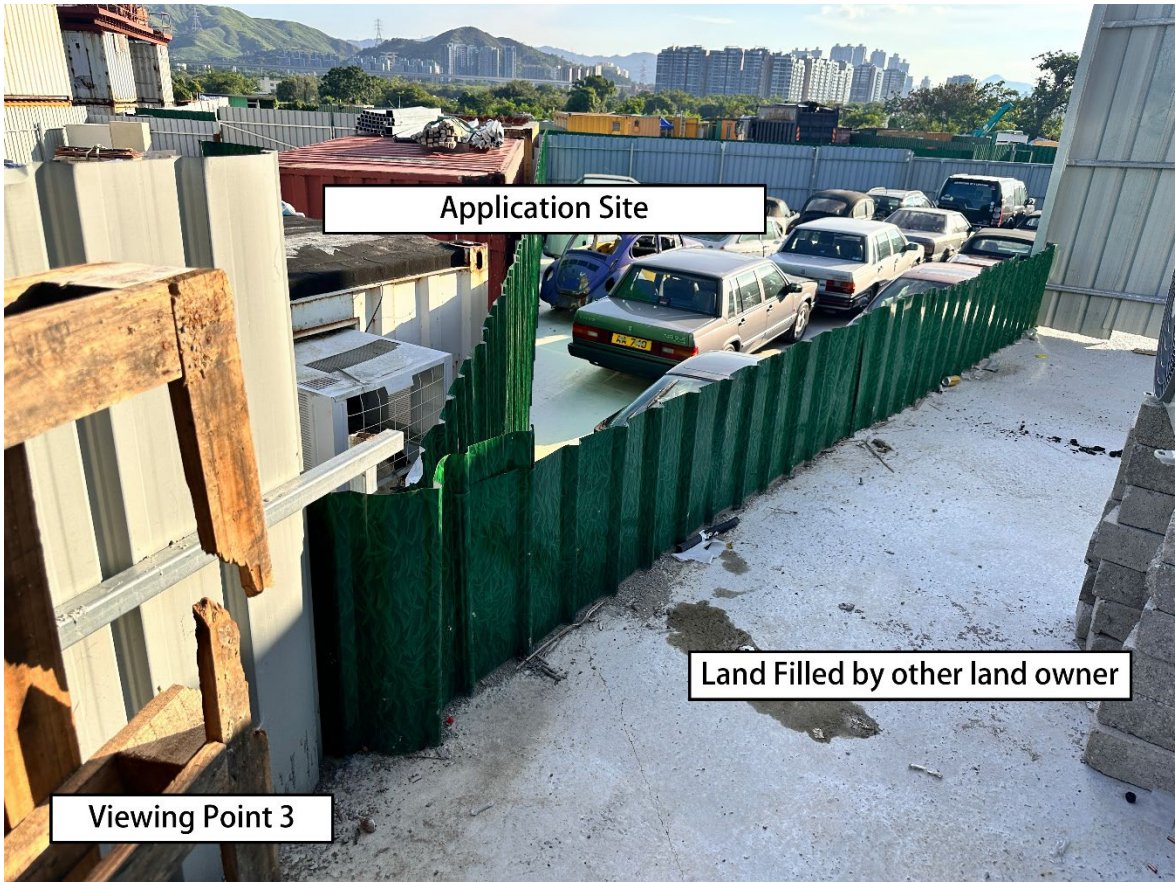
- Existing Channel
- Application Site

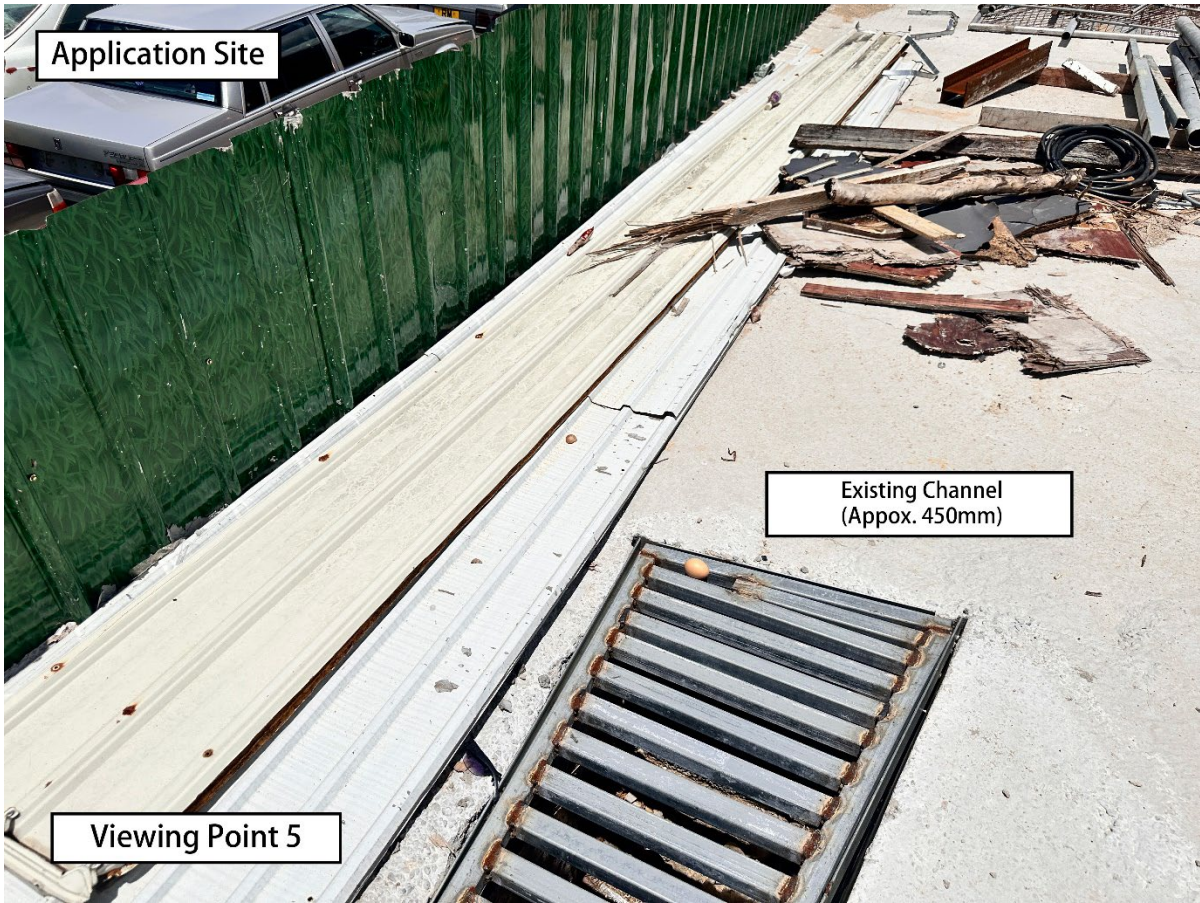
Captured from hkmappointments ib5000 2-SE-C and ib5000 6-NE-A on 6th July 2024

Appendix 5.5	Location: DD 107 Lot 1247 (Part), 1248 (Part), 1249 (Part), 1252 (Part) and 1253 (Part) OZP: S/YL-KTN/11 District: Kam Tin North Zoning: Agriculture	Project: Proposed Temporary Warehouse (excluding Dangerous Goods Godown) with Ancillary Facilities for a Period of 3 Years and Filling of Land	<u>Ultimate Discharge Point</u> (From Application Site to Nullah)	Scale: Undefined @A4	Drawing No. 5.5-1 For Identification Only Date: 19 July 2024

現場相片







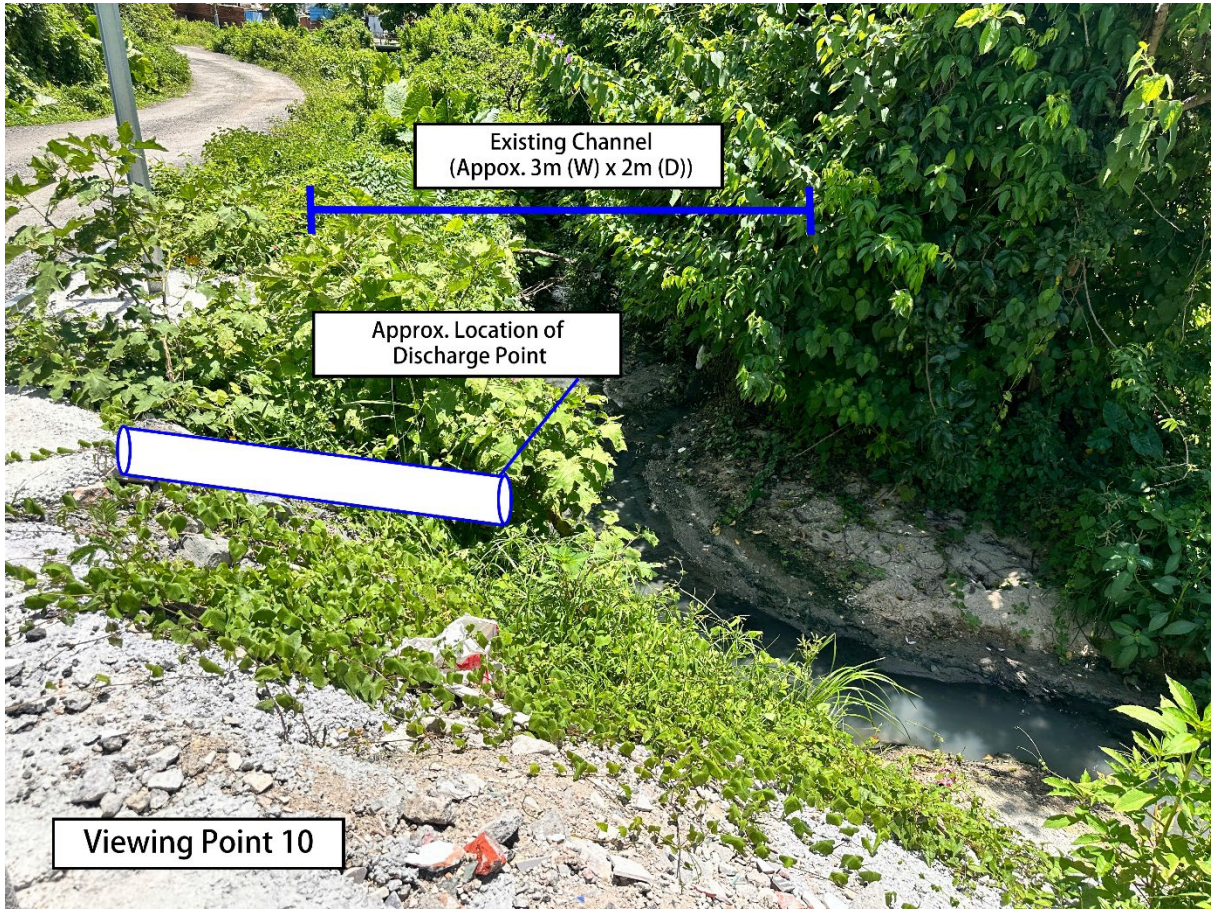




Viewing Point 9

Existing Channel
(Approx. 300mm)

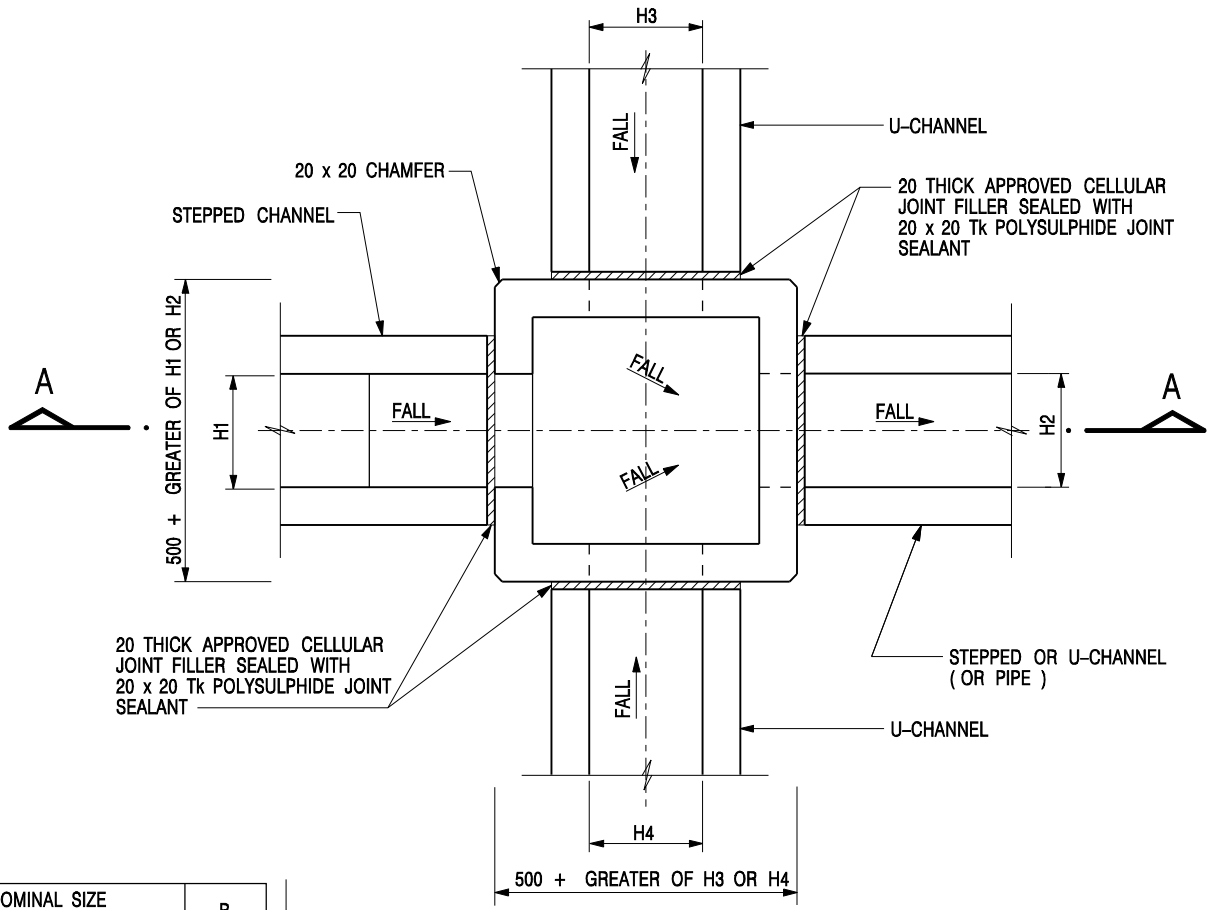




Existing Channel
(Approx. 3m (W) x 2m (D))

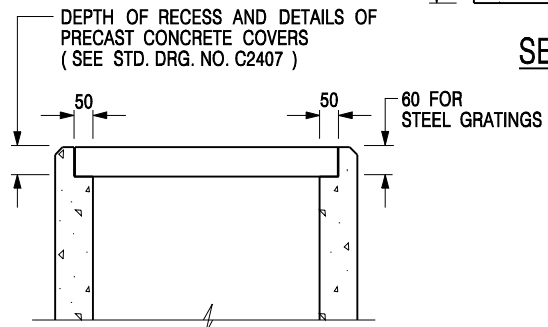
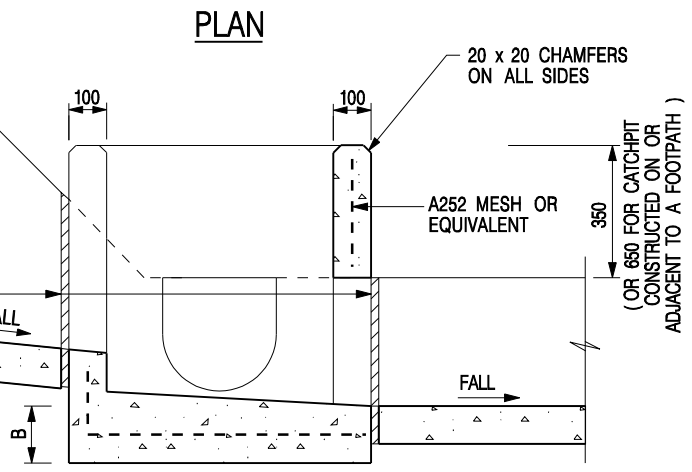
Approx. Location of
Discharge Point

Viewing Point 10



NOMINAL SIZE (LARGEST OF H1, H2, H3 & H4)	B
300 - 600	150
675 - 900	175

20 THICK APPROVED CELLULAR JOINT FILLER SEALED WITH 20 x 20 Tk POLYSULPHIDE JOINT SEALANT

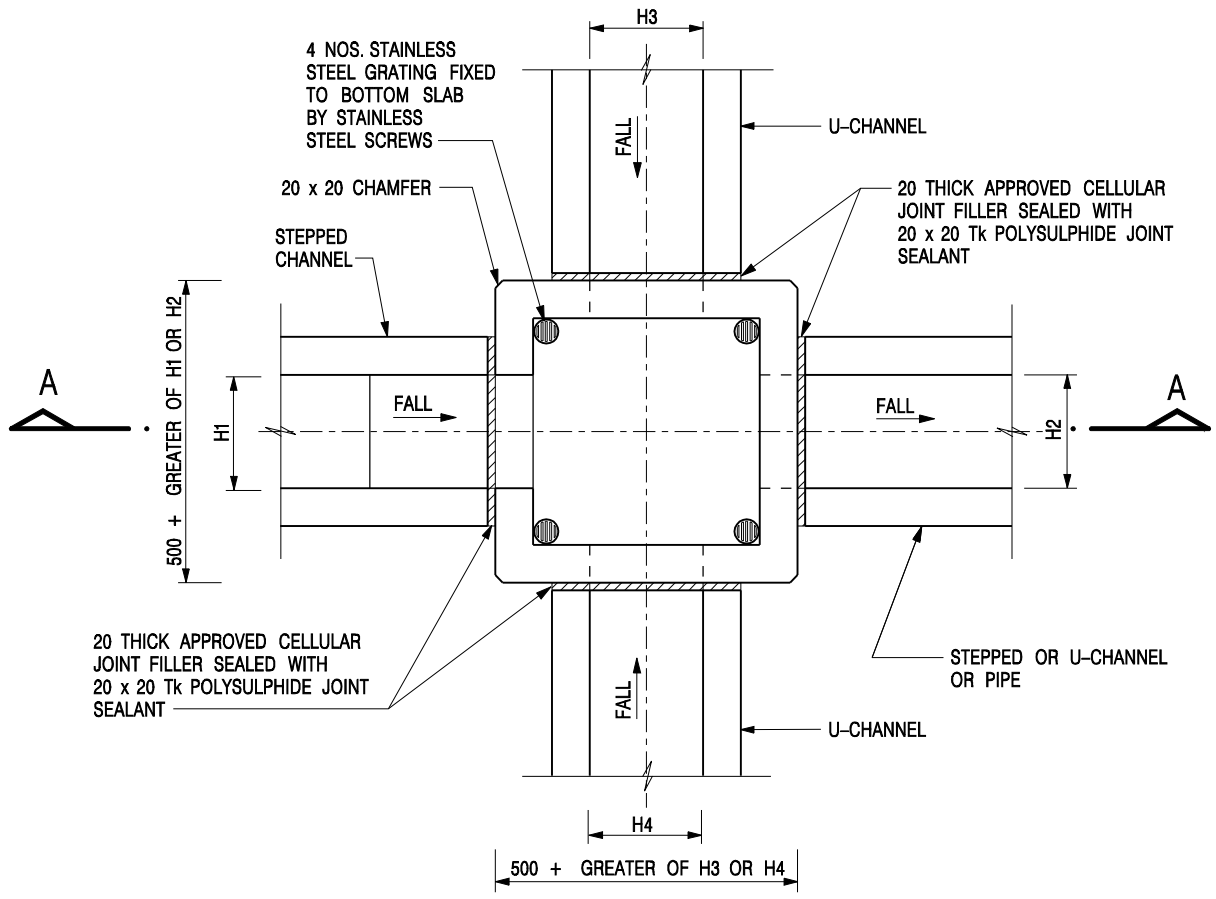


- NOTES:**
1. ALL DIMENSIONS ARE IN MILLIMETRES.
 2. REFER TO SHEET 5 FOR OTHER NOTES.

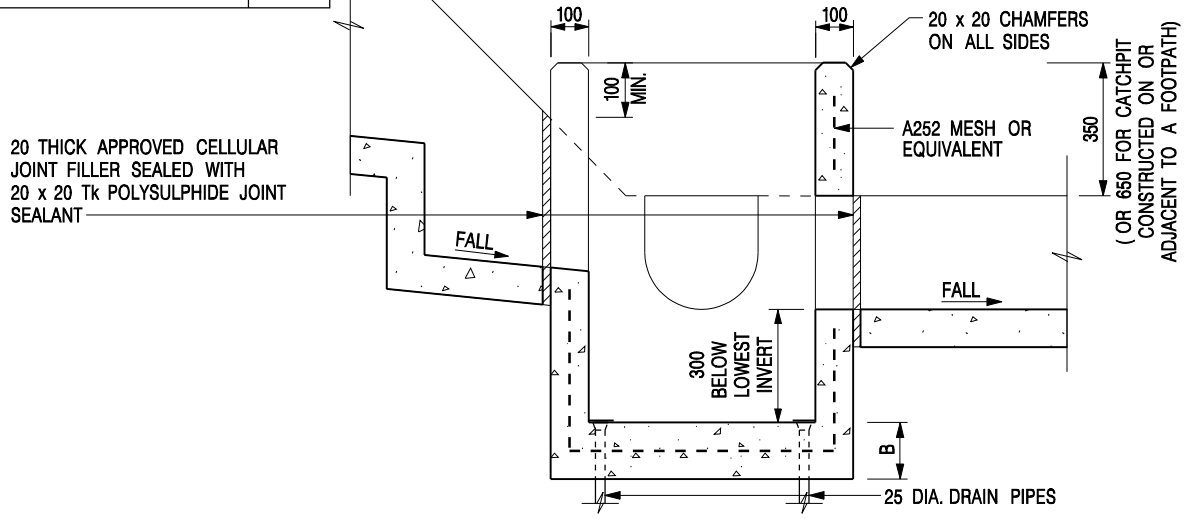
ALTERNATIVE TOP SECTION FOR PRECAST CONCRETE COVERS / GRATINGS

STANDARD CATCHPIT DETAILS
(SHEET 1 OF 5)

-	FORMER DRG. NO. C2405J.	Original Signed	03.2015
REF.	REVISION	SIGNATURE	DATE
CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT		SCALE 1 : 20 DATE JAN 1991	



NOMINAL SIZE (LARGEST OF H1, H2, H3 & H4)	B
300 - 600	150
675 - 900	175



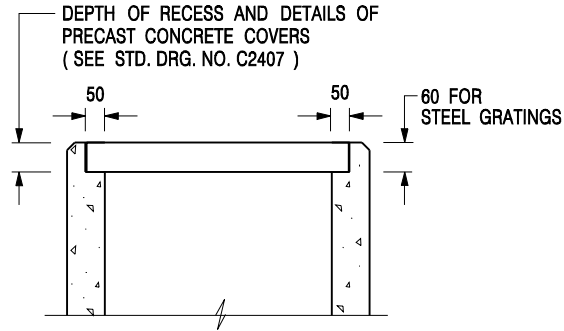
- NOTES:**
1. ALL DIMENSIONS ARE IN MILLIMETRES.
 2. REFER TO SHEET 2 FOR OTHER NOTES.

-	FORMER DRG. NO. C2406J.	Original Signed	03.2015
REF.	REVISION	SIGNATURE	DATE

CATCHPIT WITH TRAP
(SHEET 1 OF 2)

CEDD **CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT**

SCALE 1 : 20 **DRAWING NO.** C2406 /1
DATE JAN 1991




**ALTERNATIVE TOP SECTION
FOR PRECAST CONCRETE COVERS / GRATINGS**

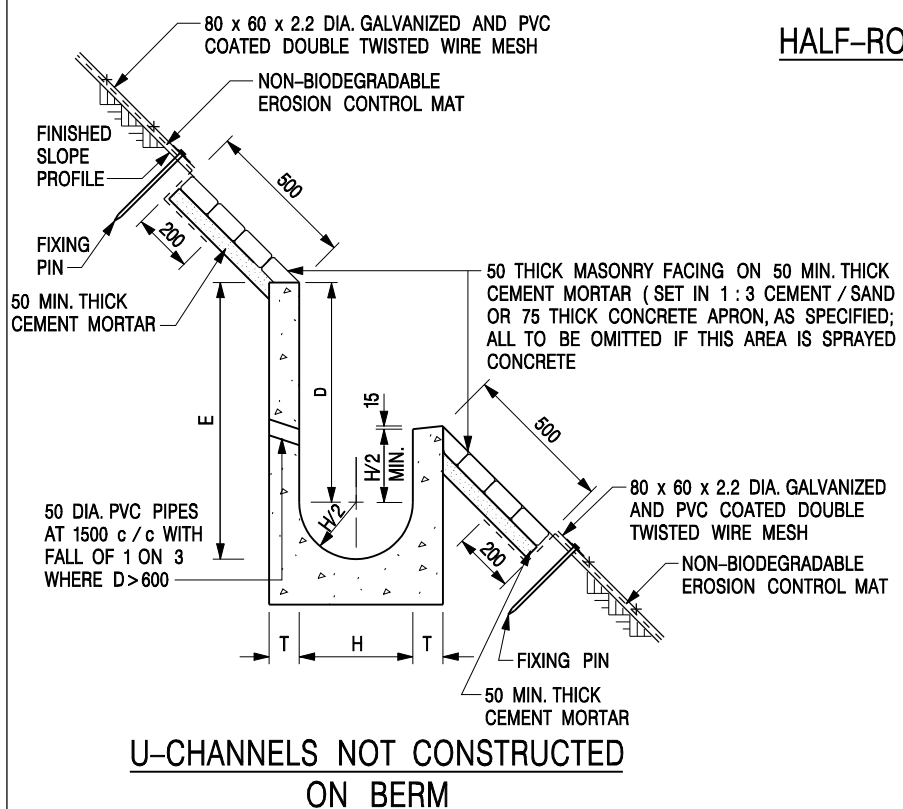
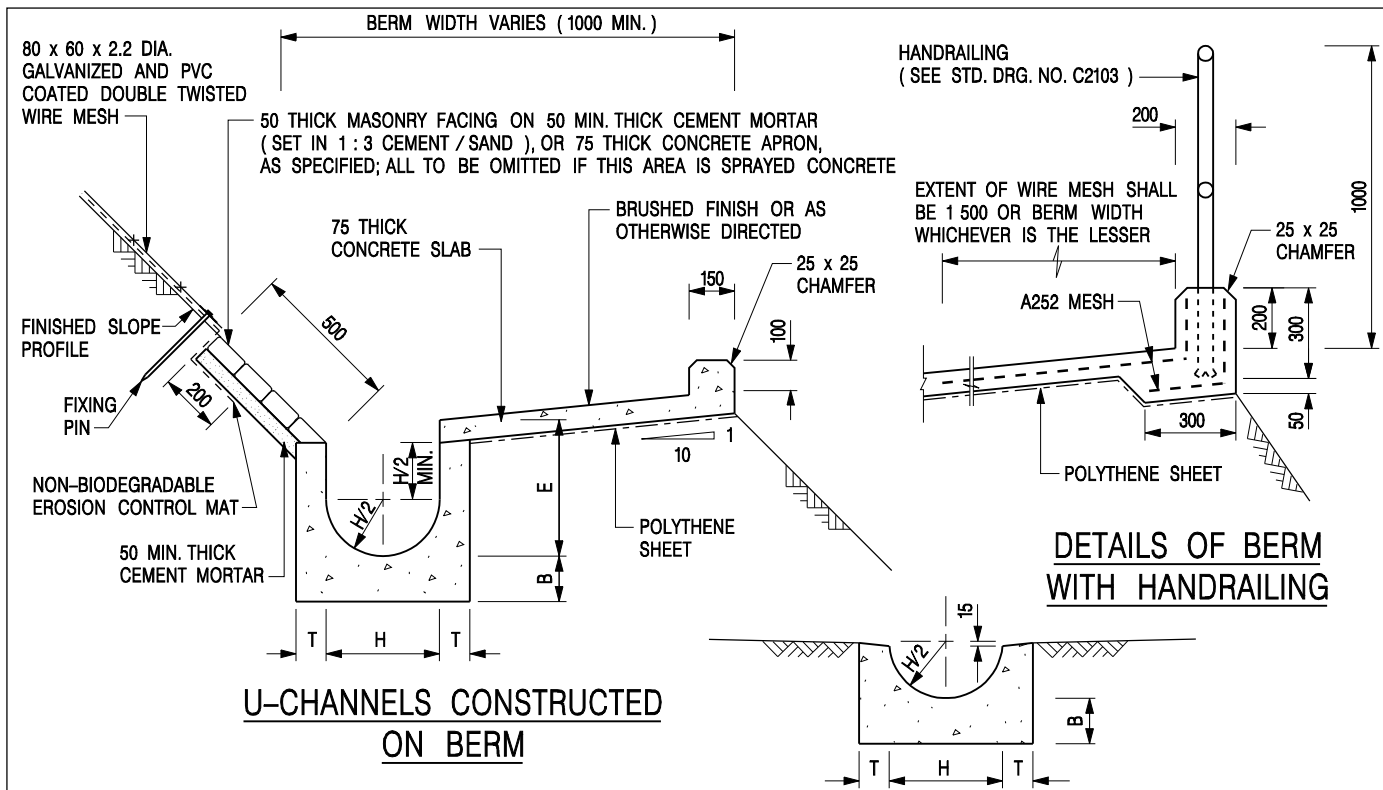
NOTES:

1. ALL DIMENSIONS ARE IN MILLIMETRES.
2. ALL CONCRETE SHALL BE GRADE 20 /20.
3. CONCRETE SURFACE FINISH SHALL BE CLASS U2 OR F2 AS APPROPRIATE.
4. FOR DETAILS OF JOINT, REFER TO STD. DRG. NO. C2413.
5. CONCRETE TO BE COLOURED AS SPECIFIED.
6. UNLESS REQUESTED BY THE MAINTENANCE PARTY AND AS DIRECTED BY THE ENGINEER, CATCHPIT WITH TRAP IS NORMALLY NOT PREFERRED DUE TO PONDING PROBLEM.
7. UPON THE REQUEST FROM MAINTENANCE PARTY, DRAIN PIPES AT CATCHPIT BASE CAN BE USED BUT THIS IS FOR CATCHPITS LOCATED AT SLOPE TOE ONLY AND AS DIRECTED BY THE ENGINEER.
8. FOR CATCHPITS CONSTRUCTED ON OR ADJACENT TO A FOOTPATH, STEEL GRATINGS (SEE DETAIL 'A' ON STD. DRG. NO. C2405 /2) OR CONCRETE COVERS (SEE STD. DRG. NO. C2407) SHALL BE PROVIDED AS DIRECTED BY THE ENGINEER.
9. IF INSTRUCTED BY THE ENGINEER, HANDRAILING (SEE DETAIL 'J' ON STD. DRG. NO. C2405 /5; EXCEPT ON THE UPSLOPE SIDE) IN LIEU OF STEEL GRATINGS OR CONCRETE COVERS CAN BE ACCEPTED AS AN ALTERNATIVE SAFETY MEASURE FOR CATCHPITS NOT ON A FOOTPATH NOR ADJACENT TO IT. TOP OF THE HANDRAILING SHALL BE 1 000 mm MIN. MEASURED FROM THE ADJACENT GROUND LEVEL.
10. MINIMUM INTERNAL CATCHPIT WIDTH SHALL BE 1 000 mm FOR CATCHPITS WITH A HEIGHT EXCEEDING 1 000 mm MEASURED FROM THE INVERT LEVEL TO THE ADJACENT GROUND LEVEL. AND, STEP IRONS (SEE DSD STD. DRG. NO. DS1043) AT 300 c/c STAGGERED SHALL BE PROVIDED. THICKNESS OF CATCHPIT WALL FOR INSTALLATION OF STEP IRONS SHALL BE INCREASED TO 150 mm.
11. FOR RETROFITTING AN EXISTING CATCHPIT WITH STEEL GRATING, SEE DETAIL 'G' ON STD. DRG. NO. C2405 /4.
12. SUBJECT TO THE APPROVAL OF THE ENGINEER, OTHER MATERIALS CAN ALSO BE USED AS COVERS / GRATINGS.

A	MINOR AMENDMENT.	Original Signed	04.2016
-	FORMER DRG. NO. C2406J.	Original Signed	03.2015
REF.	REVISION	SIGNATURE	DATE

**CATCHPIT WITH TRAP
(SHEET 2 OF 2)**

 CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT	
SCALE 1 : 20	DRAWING NO. C2406 /2A
DATE JAN 1991	



NOTES:

1. ALL DIMENSIONS ARE IN MILLIMETRES.
2. ALL CONCRETE TO BE GRADE 20 / 20.
3. CONCRETE SURFACE FINISH SHALL BE CLASS U2, F2 OR BRUSHED FINISH AS DIRECTED.
4. SPACING OF EXPANSION JOINT IN CHANNELS, BERM SLABS AND APRONS TO BE 10 METRES MAXIMUM, SEE STD. DRG. NO. C2413 FOR DETAILS.
5. JOINTS FOR CHANNELS, BERM SLABS, APRONS AND WALLS, ETC. TO BE ON THE SAME ALIGNMENT.
6. FOR DIMENSIONS T, H, & B, SEE TABLE BELOW.
7. BIODEGRADABLE EROSION CONTROL MAT IF REQUIRED, SEE STD. DRG. NO. C2511/E.
8. CONCRETE TO BE COLOURED AS SPECIFIED.
9. CONCRETE U-CHANNEL CAN BE CAST IN-SITU OR PRECAST CONCRETE SUBJECT TO THE ENGINEER'S AGREEMENT ON THE DETAILS.
10. DETAILS OF EROSION CONTROL MAT AND WESH MESH ON BERM. (SEE STD DRG. NO. C2511/E)

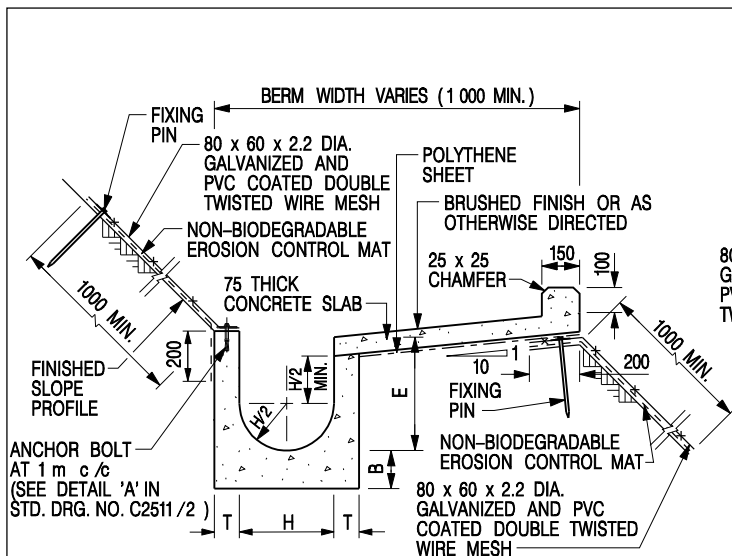
NOMINAL SIZE H	T	B	REINFORCEMENT
300	80	100	A252 MESH PLACED CENTRALLY AND T=100 WHEN E>650
375 - 600	100	150	
675 - 900	125	175	A252 MESH PLACED CENTRALLY

REF.	REVISION	SIGNATURE	DATE
I	MINOR AMENDMENT.	Original Signed	07.2018
H	THICKNESS OF MASONRY FACING AMENDED.	Original Signed	01.2005
G	MINOR AMENDMENT.	Original Signed	01.2004
F	GENERAL REVISION.	Original Signed	12.2002
E	DRAWING TITLE AMENDED.	Original Signed	11.2001
D	MINOR AMENDMENT.	Original Signed	08.2001
C	150 x 100 UPSTAND ADDED AT BERM.	Original Signed	6.99
B	MINOR AMENDMENTS.	Original Signed	3.94

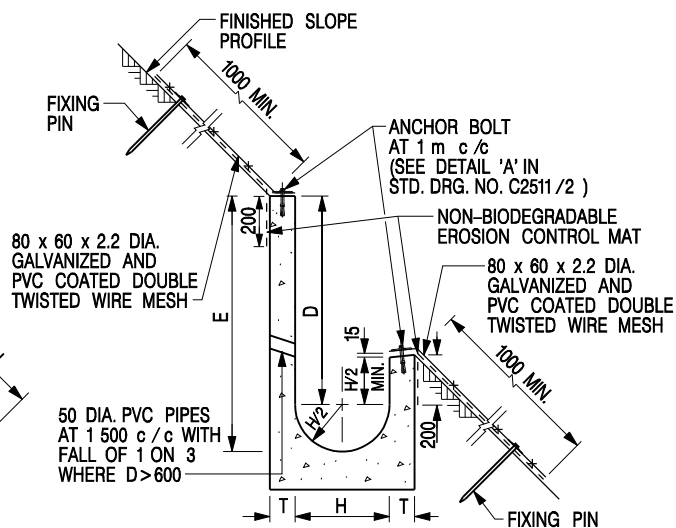
DETAILS OF HALF-ROUND AND U-CHANNELS (TYPE A - WITH MASONRY APRON)

CEDD CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

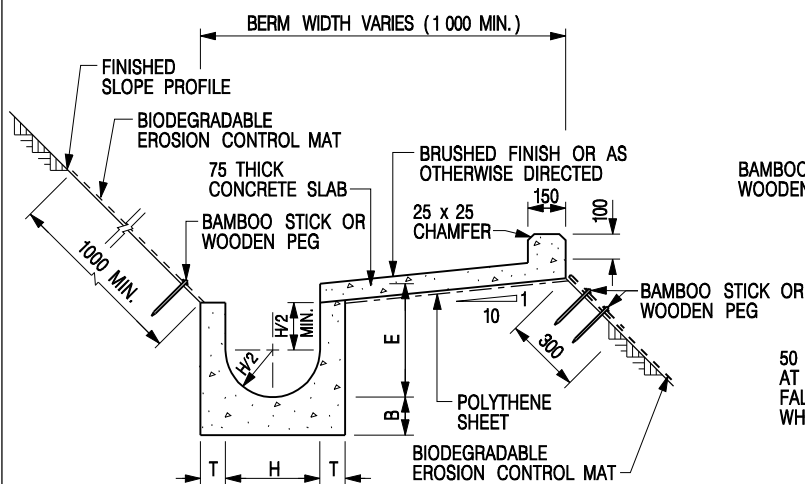
SCALE 1 : 25 **DRAWING NO. C2409I**
DATE JAN 1991



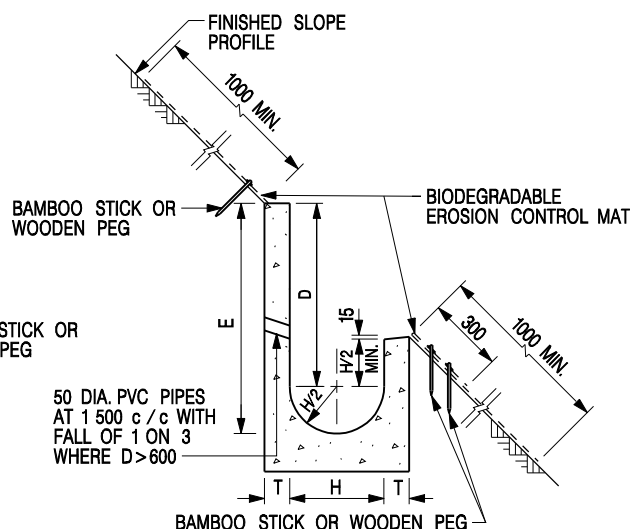
U-CHANNELS CONSTRUCTED ON BERM WITH NON-BIODEGRADABLE EROSION CONTROL MAT



U-CHANNELS NOT CONSTRUCTED ON BERM WITH NON-BIODEGRADABLE EROSION CONTROL MAT



U-CHANNELS CONSTRUCTED ON BERM WITH BIODEGRADABLE EROSION CONTROL MAT



U-CHANNELS NOT CONSTRUCTED ON BERM WITH BIODEGRADABLE EROSION CONTROL MAT

NOTES:

- ALL DIMENSIONS ARE IN MILLIMETRES.
- ALL CONCRETE TO BE GRADE 20 /20.
- CONCRETE SURFACE FINISH SHALL BE CLASS U2, F2 OR BRUSHED FINISH AS DIRECTED.
- SPACING OF EXPANSION JOINT IN CHANNELS, BERM SLABS AND APRONS TO BE 10 METRES MAXIMUM, SEE STD. DRG. NO. C2413 FOR DETAILS.
- JOINTS FOR CHANNELS, BERM SLABS, APRONS AND WALLS, ETC. TO BE ON THE SAME ALIGNMENT.
- FOR DIMENSIONS T, H, & B, SEE TABLE BELOW.
- FOR TYPICAL FIXING PIN DETAILS, SEE STD. DRG. NO. C2511/2.
- MINIMUM SIZE OF 25 x 50 x 300mm SHALL BE PROVIDED FOR WOODEN PEG.
- MINIMUM SIZE OF 10mm DIAMETER WITH 200mm LONG SHALL BE PROVIDED FOR BAMBOO STICK.
- THE FIXING DETAILS OF NON-BIODEGRADABLE AND BIODEGRADABLE EROSION CONTROL MATS ON EXISTING BERM SHALL REFER TO STD. DRG. NO. C2511/1.

NOMINAL SIZE H	T	B	REINFORCEMENT
300	80	100	A252 MESH PLACED CENTRALLY AND T=100 WHEN E > 650
375 - 600	100	150	
675 - 900	125	175	

REF.	REVISION	SIGNATURE	DATE
I	MINOR AMENDMENT.	Original Signed	07.2018
H	FIXING DETAILS OF BIODEGRADABLE EROSION CONTROL MAT ADDED.	Original Signed	12.2017
G	DIMENSION TABLE AMENDED.	Original Signed	01.2005
F	MINOR AMENDMENT.	Original Signed	01.2004
E	GENERAL REVISION.	Original Signed	12.2002
D	MINOR AMENDMENT.	Original Signed	08.2001
C	150 x 100 UPSTAND ADDED AT BERM.	Original Signed	6.99
B	MINOR AMENDMENT.	Original Signed	3.94
A	MINOR AMENDMENT.	Original Signed	10.92

DETAILS OF HALF-ROUND AND U-CHANNELS (TYPE B - WITH EROSION CONTROL MAT APRON)



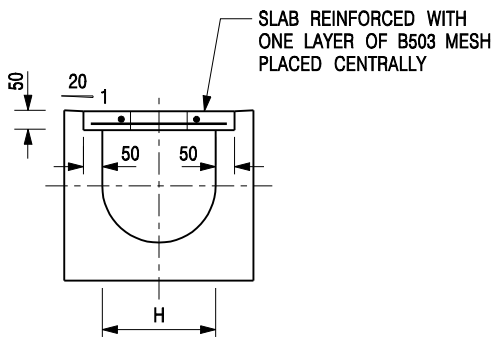
CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT

SCALE DIAGRAMMATIC

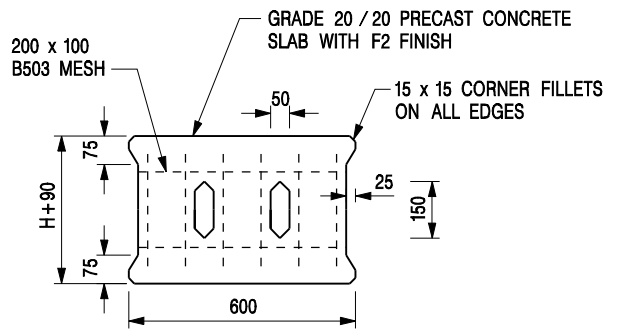
DRAWING NO.

DATE JAN 1991

C24101



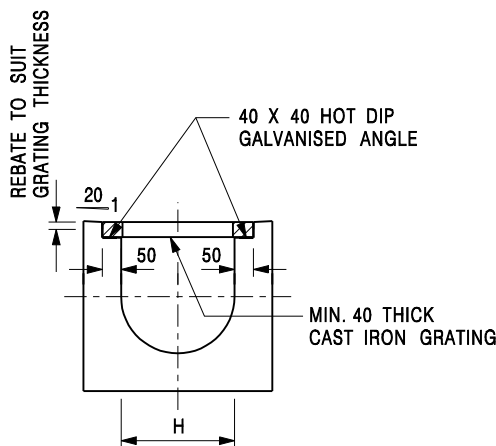
TYPICAL SECTION



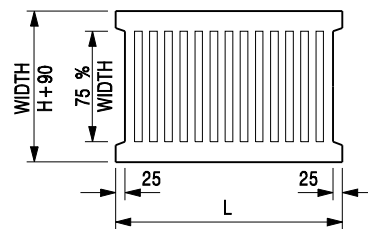
PLAN OF SLAB

U-CHANNELS WITH PRECAST CONCRETE SLABS

(UP TO H OF 525)



TYPICAL SECTION



L = 600mm FOR H ≤ 375mm
L = 400mm FOR H > 375mm

CAST IRON GRATING

(DIMENSIONS ARE FOR GUIDANCE ONLY, CONTRACTOR MAY SUBMIT EQUIVALENT TYPE)

U-CHANNEL WITH CAST IRON GRATING

(UP TO H OF 525)

NOTES:

1. ALL DIMENSIONS ARE IN MILLIMETRES.
2. H=NOMINAL CHANNEL SIZE.
3. ALL CAST IRON FOR GRATINGS SHALL BE GRADE EN-GJL-150 COMPLYING WITH BS EN 1561.
4. FOR COVERED CHANNELS TO BE HANDED OVER TO HIGHWAYS DEPARTMENT FOR MAINTENANCE, THE GRATING DETAILS SHALL FOLLOW THOSE AS SHOWN ON HyD STD. DRG. NO. H3156.

E	NOTES 3 & 4 AMENDED.	Original Signed	12.2014
D	NOTE 4 ADDED.	Original Signed	06.2008
C	MINOR AMENDMENT. NOTE 3 ADDED.	Original Signed	12.2005
B	NAME OF DEPARTMENT AMENDED.	Original Signed	01.2005
A	CAST IRON GRATING AMENDED.	Original Signed	12.2002
REF.	REVISION	SIGNATURE	DATE

**COVER SLAB AND CAST IRON
GRATING FOR CHANNELS**



**CIVIL ENGINEERING AND
DEVELOPMENT DEPARTMENT**

SCALE 1 : 20

DRAWING NO.

DATE JAN 1991

C2412E

Table 3a – Storm Constants for Different Return Periods of HKO Headquarters

Return Period T (years)	2	5	10	20	50	100	200	500	1000
a	499.8	480.2	471.9	463.6	451.3	440.8	429.5	414.0	402.1
b	4.26	3.36	3.02	2.76	2.46	2.26	2.05	1.77	1.55
c	0.494	0.429	0.397	0.369	0.337	0.316	0.295	0.269	0.251

Table 3b – Storm Constants for Different Return Periods of Tai Mo Shan Area

Return Period T (years)	2	5	10	20	50	100	200
a	1743.9	2183.2	2251.3	2159.2	1740.1	1307.3	1005.0
b	22.12	27.12	27.46	25.79	19.78	12.85	7.01
c	0.694	0.682	0.661	0.633	0.570	0.501	0.434

Table 3c – Storm Constants for Different Return Periods of West Lantau Area

Return Period T (years)	2	5	10	20	50	100	200
a	2047.9	1994.1	1735.2	1445.6	1107.2	909.1	761.8
b	24.27	24.23	21.82	18.36	13.01	8.98	5.40
c	0.733	0.673	0.619	0.561	0.484	0.428	0.377

Table 3d – Storm Constants for Different Return Periods of North District Area

Return Period T (years)	2	5	10	20	50	100	200
a	1004.5	1112.2	1157.7	1178.6	1167.6	1131.2	1074.8
b	17.24	18.86	19.04	18.49	16.76	14.82	12.47
c	0.644	0.614	0.597	0.582	0.561	0.543	0.523

Table 13 - Values of n to be used with the Manning equation

Source: Brater, E.F. & King, H.W. (1976)

Surface	Best	Good	Fair	Bad
Uncoated cast-iron pipe	0.012	0.013	0.014	0.015
Coated cast-iron pipe	0.011	0.012*	0.013*	
Commercial wrought-iron pipe, black	0.012	0.013	0.014	0.015
Commercial wrought-iron pipe, galvanized	0.013	0.014	0.015	0.017
Smooth brass and glass pipe	0.009	0.010	0.011	0.013
Smooth lockbar and welded "OD" pipe	0.010	0.011*	0.013*	
Riveted and spiral steel pipe	0.013	0.015*	0.017*	
Vitrified sewer pipe	0.010	0.013*	0.015	0.017
Common clay drainage tile	0.011	0.012*	0.014*	0.017
Glazed brickwork	0.011	0.012	0.013*	0.015
Brick in cement mortar; brick sewers	0.012	0.013	0.015*	0.017
Neat cement surfaces	0.010	0.011	0.012	0.013
Cement mortar surfaces	0.011	0.012	0.013*	0.015
Concrete pipe	0.012	0.013	0.015*	0.016
Wood stave pipe	0.010	0.011	0.012	0.013
Plank flumes - Planed	0.010	0.012*	0.013	0.014
- Unplaned	0.011	0.013*	0.014	0.015
- With battens	0.012	0.015*	0.016	
Concrete-lined channels	0.012	0.014*	0.016*	0.018
Cement-rubble surface	0.017	0.020	0.025	0.030
Dry-rubble surface	0.025	0.030	0.033	0.035
Dressed-ashlar surface	0.013	0.014	0.015	0.017
Semicircular metal flumes, smooth	0.011	0.012	0.013	0.015
Semicircular metal flumes, corrugated	0.0225	0.025	0.0275	0.030
Canals and ditches				
1. Earth, straight and uniform	0.017	0.020	0.0225*	0.025
2. Rock cuts, smooth and uniform	0.025	0.030	0.033*	0.035
3. Rock cuts, jagged and irregular	0.035	0.040	0.045	
4. Winding sluggish canals	0.0225	0.025*	0.0275	0.030
5. Dredged-earth channels	0.025	0.0275*	0.030	0.033
6. Canals with rough stony beds, weeds on earth banks	0.025	0.030	0.035*	0.040
7. Earth bottom, rubble sides	0.028	0.030*	0.033*	0.035
Natural-stream channels				
1. Clean, straight bank, full stage, no rifts or deep pools	0.025	0.0275	0.030	0.033
2. Same as (1) but some weeds and stones	0.030	0.033	0.035	0.040
3. Winding some pools and shoals, clean	0.033	0.035	0.040	0.045
4. Same as (3), lower stages, more ineffective slope and sections	0.040	0.045	0.050	0.055

Table 13 (Cont'd)

Surface	Best	Good	Fair	Bad
5. Same as (3) some weeds and stones	0.035	0.040	0.045	0.050
6. Same as (4) stony sections	0.045	0.050	0.055	0.060
7. Sluggish river reach, rather weedy or with very deep pools	0.050	0.060	0.070	0.080
8. Very weedy reaches	0.075	0.100	0.125	0.150

Notes: *Values commonly used for design.